



## Impacts of combining meteorological and hydrometric data on the accuracy of streamflow modeling

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### Abstract

Proper modeling of rainfall-runoff is essential for water quantity and quality management. However, comprehensive evaluation of soft computing techniques for rainfall-runoff modeling in developing countries is still lacking. Towards this end, in the present study two new soft computing techniques of genetic programming (GP) and M5 model tree were formulated for daily streamflow prediction. Firstly, the daily meteorological and hydrometric data including rainfall, temperature, evapotranspiration, relative humidity and discharge were collected for the years 1970 - 2012 throughout Amameh Watershed in Tehran, Iran. Secondly, the input variables were determined using cross-correlation and then 62 different scenarios were developed. Thirdly, the data were standardized in the range of zero to one. Finally, performance of the scenarios was assessed using the mean square error (MSE), root mean square error (RMSE) and mean absolute error (MAE). Totally, 80 and 20 percent of instances were used for training and testing, respectively. The results showed that the scenario number 54 was the best using both GP and M5 model tree techniques. However, GP showed much better performance than M5 model tree with MSE, RMSE, and MAE values of 0.001, 0.031 and 0.009 for training and 0.001, 0.032 and 0.009 for testing, respectively. The scenario 54 had eight inputs including rainfall, discharge, and delay for two days, temperature, evapotranspiration and relative humidity.

**Keywords:** Genetic programming, Model development, M5 model tree, Scenario analysis, Streamflow prediction

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## Introduction

Understanding the governing properties of watershed hydrology under different conditions is a challenging issue throughout the globe (Huo et al., 2012; Danandeh Mehr et al., 2013; Moatamednia et al., 2015; Ghorbani et al., 2018). Numerous factors including climate condition, vegetation cover, soil infiltration, and land use affect the relationship of hydrological process and particularly rainfall-runoff processes (Keshtegar et al., 2018; Rezaie-Balf et al., 2019). To optimally design and operate water resources structures or to appropriately plan structures use and maintenance we need to have detailed information on rainfall-runoff relationships in a particular time interval or period (Huo et al., 2012; Danandeh Mehr et al., 2013; Moatamednia et al., 2015; Rezaie-Balf and Kisi, 2017). The increasing development trend in computational intelligence field has led to appearance and development of computer and technology-based rainfall-runoff models (Solaimani, 2009; Huo et al., 2012; Chandwani, et al., 2015; Liu et al., 2017; Najafzadeh et al., 2018; Lu et al., 2018). In this field, rainfall-runoff models could be very helpful for flood control measures, drought management and water supply allocation. On the other hands, basic information for river flow forecasting is needed to provide solutions to a wide range of problems related to the design and operation of river systems. The availability of rainfall records and other meteorological data, which can be used to obtain streamflow data, initiated the practice of rainfall-runoff modeling (Behzad et al., 2009). In addition, it is well documented that the rainfall-runoff models have important role in water resource management planning, irrigation and water supply. Different models with various degrees of complexity have been developed (Dooge, 1977; Harun et al., 2002; Bhattacharya and Solomatine, 2005; Solaimani, 2009; Besaw et al., 2010; Fernando et al., 2011; Abrahart et al., 2012; Motamednia et al., 2015; Najafzadeh et al., 2016; Liu et al., 2017;

Rezaie-Balf and Kisi, 2017; Keshtegar et al., 2018; Rezaie-Balf et al., 2019).

Hydrologists are often confronted with problems of prediction and estimation of runoff, precipitation, water stages, and so on (Harun et al., 2002; Danandeh Mehr et al., 2013; Motamednia et al., 2015). System theoretic models do not consider the physical characteristics of the parameters; while they illustrate the data from input to output using transferred functions. Where the interactions among the variables are complex, the conventional mathematical techniques in the form of regression equations can't provide a perfect representation. As functional relationship of rainfall-runoff can be extremely complex (Fernando et al., 2011), today, the soft computing tools offer a simplified approach over conventional hard computing in dealing with these nature based phenomena (Chandwani et al., 2015; Danandeh Mehr, 2018). Specifically, for streamflow prediction, due to the lack of precise knowledge and the complexity of the effective factors, different models have been developed (Danandeh Mehr, 2013; Rezaie-Balf and Kisi, 2017; Rezaie-Balf et al., 2017; Keshtegar et al., 2018). Genetic programming (GP) as a clear and effective method can be used for estimation of water based data (Danandeh Mehr, 2014; Talebi et al., 2017). GP is a self-parameterizing method that build models without any user tuning (Sreekanth and Datta, 2011; Danandeh Mehr et al., 2013; Danandeh Mehr et al., 2014). A GP model is a member of the evolutionary algorithm family, which are based upon the concept of natural selection and genetic evolution (Koza, 1992; Solomatine and Xue, 2004; Guven, 2009; Gorbani et al., 2010; Wang et al., 2014; Danandeh Mehr, 2018). Genetic algorithm (GA) that was suggested by Holland (1975) was the base idea to GP development by Koza (1992). The background of GP is similar to GA which includes defining the fitness function, for instance crossover, mutation and reproduction and the termination criterion.

Crossover operator in GP is applied to change the sub-tree from the parents to reproduce the children by means of mating selection policy instead of exchanging bit strings as in GA (Wang et al., 2009). This method works with a number of solution sets collectively known as a population rather than a single solution at any time, therefore the possibility of getting trapped in a local optimum is avoided. However, GP is different from traditional GA. in that it typically operates on parse tree instead of bit string. A parse tree is built up from a terminal set (the input variables in the problem and randomly generated constants, i.e. empirical model coefficients) and a function set, hence the basic operators are used to form the GP model. The next is user-defined and can not only include algebraic operators such as {+, -, \*, /, exp., sin} but also take the form of logical rules, making use of operators such as {IF, OR, AND} (Selle and Muttill, 2011).

Ghorbani et al. (2010) reported the high performance of GP in comparison with neural networks (ANN) and neuro-fuzzy (FIS) methods of flood routing of Kizilirmak River, Turkey. Ajmera and Goyal (2012) also investigated modeling of the stage–discharge in Peachtree Creek in Atlanta using ANN and M5 model tree and then the results were compared with discharge from current and traditional methods. Their results showed that, M5 model performed better than ANN. Danandeh Mehr (2013) used wavelet-ANN (WANN) and linear GP (LGP) to predict river flow on a monthly scale. The GP, WANN and three-layer perceptron neural network techniques based on the statistical evaluation showed good performance. Additionally, an explicit LGP model constructed by only basic arithmetic functions including one month-lagged records of both target and upstream stations revealed the best prediction model for the studied river. Sattari et al. (2013) surveyed the potential of Multi-Layer Perceptron (MLP) with back propagation algorithm and M5 model tree based regression approaches to model monthly reference evapotranspiration using

climatic data of an area around Ankara, Turkey. The results revealed that the M5 model tree, could predict river flow better than the support vector machine (SVM). It is also concluded that a simple linear relationships using M5 model tree requires less computational time. Meshgi et al. (2015) used GP for streamflow modeling in different land uses of Kent Ridge Watershed, southern part of Singapore. Al-Juboori and Guven (2016) worked on GEP-based monthly streamflow forecasting model for perennial rivers in Hurman River in Turkey as well as Diyalah and Lesser Zab Rivers in Iraq. They divided monthly flow data into 12 intervals because of the number of months in a year. The result showed the importance of seasonality effect in the selection of potential predictors which is the major pattern in the intermittent streamflow series. Keshtegar et al. (2018) compared four heuristic regression techniques such as Kriging method vs. RSM, multivariate adaptive regression spline (MARS) and M5 model tree in Adana and Antakya stations located in Eastern Mediterranean Region of Turkey. Their results showed that models M5 model tree produce inaccurate results for both maximum error and minimum agreement compared to other models. Finally, the periodic Kriging models performed superior to the periodic MARS, RSM and M5Tree models. Rezaie-Balf et al. (2019) M5 model tree (M5Tree) and MARS models to forecast river flow. They developed models using two different meteorological stations Kordkheyl in Iran and Hongcheon in South Korea. Results showed that EEMD-MARS model was an efficient and robust tool to forecast one and multi-day-ahead like two, three, and four-day-ahead river flow. The GP model is very similar to GA, in which each chromosome in initial population is a potential solution for a given problem. However, chromosomes in GP are represented by tree-shaped structure of a computer program that contains nodes of functions and terminals with connecting branches (Danandeh Mehr, 2018). Generally, the GP process could be

completed in three steps. (1) An initial population is generated randomly for parse tree. (2) Any member of the mentioned population is considered using the fitness function for selecting better parse trees for production of new population (Gorbani et al., 2010; Wang et al., 2014). The root mean square error (RMSE) is used to meet the goal of this step, (3). In every generation, below stages are followed for population selection:

(a) One of the operators such as crossover, mutation is selected, (b) Appropriate number of available population is then chosen, (c) The selected operators are used to produce offspring, (d) These children are entered in a new

population, and (f) Models are evaluated by different fitting criteria. (4) The third step is repeated until the maximum number of generations is reached (Sreekanth and Datta, 2011; Danandeh Mehr et al., 2013). As shown in Figure 1, in GP modeling, there are functions and terminals chosen randomly from the user defined sets to form a computer model in a tree-like structure with a root node and branches extending from each function and ending in a leaf or a terminal. In many cases, in GP leaves are the inputs to the program (Koza, 1992; Babovic and Keijzer, 2000; Al-Juboori and Guven, 2016; Danandeh Mehr, 2018).

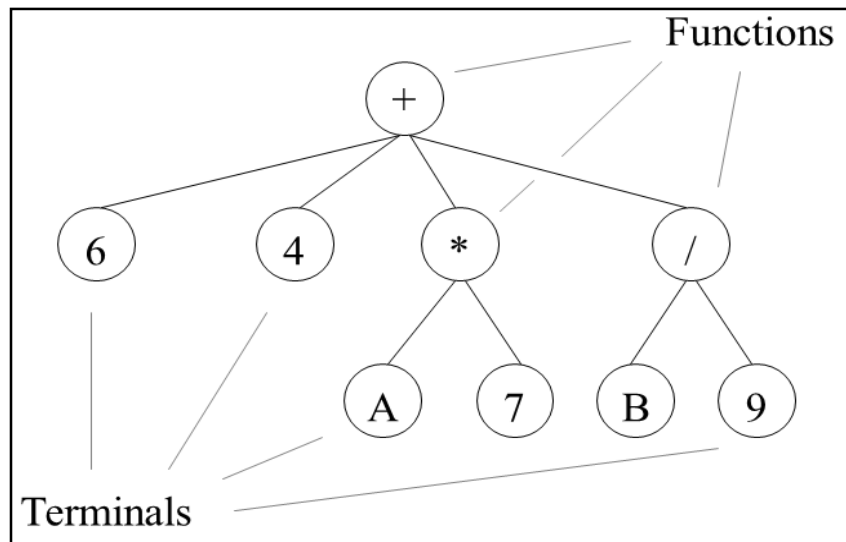


Figure 1. GP parse tree representing function  $(+ 6 4 (* A 7) (/ B 9))$

The M5 tree models are introduced as new soft computing method and generalizing the concepts of regression trees, which have constant values at their leaves (Witten and Frank, 2005; Sattari et al., 2013; Al-Juboori and Guven, 2016). M5 model tree is very capable and efficient. Today, the M5 model tree are used in various fields of hydrology and water resources, particularly in matters of classification and prediction. The M5 model trees are analogous to piece-wise linear functions (and hence nonlinear). The M5 model tree is a binary decision tree having linear regression functions at leaf nodes, which can predict continuous

numerical attributes (Zhang and Tsai, 2007; Rezaie-Balf et al., 2017; Rezaie-Balf et al., 2019). Tree-based models are constructed by a divide-and-conquer method. For generation of a model tree, two different stages are required. The first stage involves using a splitting criterion to create a decision tree. As a whole, a M5 model tree is a nonlinear regression and the splitting criterion for the M5 model tree algorithm is based on treating the standard deviation of the class values that reach a node as a measure of the error at that node and calculating the expected reduction in this error as a result of testing each attribute at that node (Keshtegar et

al., 2018). The formula to compute the standard deviation reduction (SDR) is as follows.

$$SDR = sd(T) - \sum \frac{|T_i|}{|T|} sd(T_i) \tag{1}$$

$$sd(T) = \sqrt{\frac{1}{N} \left( \sum_{i=1}^N y_i^2 - \frac{1}{N} \left( \sum_{i=1}^N y_i \right)^2 \right)} \tag{2}$$

Where T represents a set of examples that reach the node,  $T_i$  represents the subset of examples that have the  $i$ th outcome of the potential set, N is the number of data and sd represents the standard deviation. Because of the splitting process, child nodes have less standard deviation as compared to parent node and are thus more pure (Quinlan, 1992). After examining all the possible splits, M5 chooses the one that maximizes the expected error reduction. This division often produces a large tree-like structure that may cause over fitting. To counter the problem of over fitting, the tree must be pruned back, for example by replacing a sub tree with a leaf.

Thus, the second stage in the design of the model tree involves pruning the overgrown tree and replacing the sub trees with linear regression functions (Etemad-Shahidi and Bonakdar, 2009; Sattari et al.,

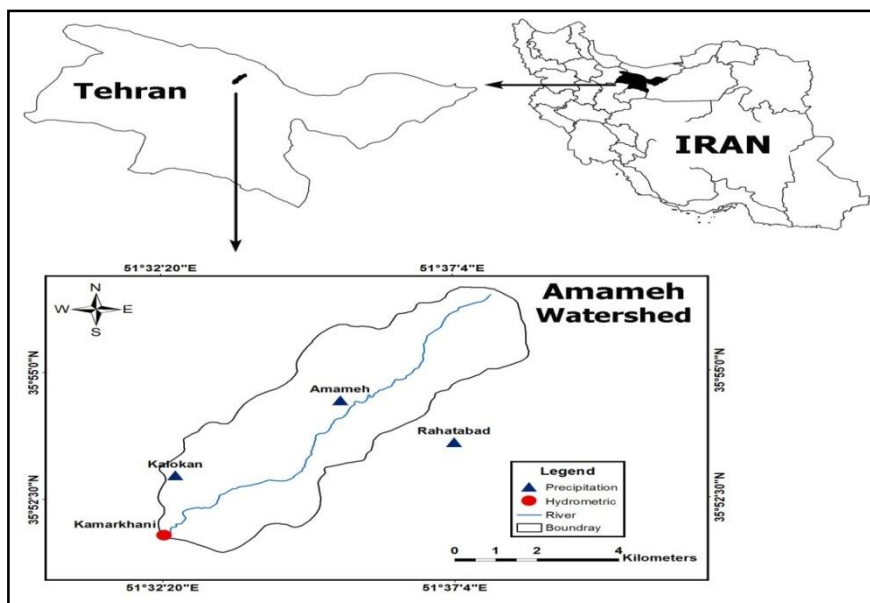
2013; Rezaie-Balf et al., 2017; Keshtegar et al., 2018; Rezaie-Balf et al., 2019). For this, the parameter space is split into subspaces and in each a linear regression model is built.

The aim of this study was to forecast river flow using two famous methods viz. GP and M5 model tree in Amameh Watershed, Iran.

**Materials and Methods**

**Study area and data**

The Amameh Watershed with an area of 37.2 km<sup>2</sup>, located in 35° 51' 50'' N-latitude and 51° 32' 27'' E-longitude is one of the sub-watersheds of the Jajroud Watershed in the southern part of central Alborz, Iran (Figure 2). The average annual temperature and rainfall are 12 °C and 350 mm, respectively. The watershed elevation ranges from 1900 to 3868 m above sea level (Nourani et al., 2009). The specific topographic features of the Amameh Watershed, the meteorological stations availability and data accessibility were the main reasons why we selected this watershed as a case study. In the present study, the daily meteorological and hydrometric data were recorded in Amameh and Kamarkhani stations located at the central and the outlet of the watershed, respectively (Figure 2).



**Figure 2.** The study area and location of the gauging stations

The meteorological and hydrometric variables were used to calculate the daily streamflow for the time period 1970-1971 to 2011-2012 (42 years). Meteorological and hydrometric parameters, namely rainfall (P), mean air temperature (T), relative humidity (Rh), evapotranspiration (ET) and discharge (Q) were considered as inputs of rainfall-runoff modeling. In the training period of both GP and M5 model tree approaches, 80 % of inputs (i.e., 1970-1971 to 2003-2004; 10944 data points for 34 years) were used. The remaining 20% of inputs (2004-2005 to 2011-2012; 2920 data points for eight

years) were used for testing of models and scenarios. Table 1 provides the statistical properties of the various meteorological and hydrometric parameters used in this study.

The data standardization was also made in order to make the data dimensionless and confine them within a certain range before entering the inputs data into training and testing steps. So, to assimilate and integrate the data, the values were normalized in the range of zero to one (Sattari et al., 2013; Danandeh Mehr et al., 2014; Motamednia et al., 2015; Danandeh Mehr, 2018).

**Table 1.** Statistical properties of used data for rainfall-runoff modeling of Amameh WatershedQ (m<sup>3</sup>/s)

Statistics	Rh (%)	ET (mm)	T (°C)	P (mm)	Q (m <sup>3</sup> /s)
Maximum	99.00	24.10	37.00	95.00	9.63
Minimum	4.00	0.00	-18.50	0.00*	0.01*
Average	51.00	3.47	10.19	1.83	0.61
Standard deviation	15.49	3.33	9.44	6.06	0.75
Coefficient of variation (%)	30.26	96.09	92.59	330.53	122.82

Rh=relative humidity, ET=evapotranspiration, T=temperature, P=rainfall, Q=discharge, \*the value of rainfall is zero but the value of discharge is 0.01 because of baseflow and interflow

One of the most important steps in the model development process is the choice of significant input variables. Although, there is no clear cut theory and rules for that but usually solutions such as cross-autocorrelation and partial autocorrelation analysis of data are used (Srinivasulu and Jain, 2006; Wu et al., 2009; Huo et al., 2012; Motamednia et al., 2015; Rezaie-Balf et al., 2017). These methods are used to reduce inputs number of variables. Cross-correlation analysis between the target streamflow Q(t) with itself and different lag time series of P, ET, Rh were performed to get the important factors for streamflow estimation. The cross-autocorrelation analysis between different inputs and their lags were also used (Huo

et al. 2012; Danandeh Mehr et al., 2014; Danandeh Mehr, 2018). According to the results of 62 scenarios and variables used as inputs with one to six time lags, inputs were selected for this study. Table 2 shows the study scenarios.

For M5 model tree, we used Weka 3.7 (Witten and Frank, 2005; Ajmera and Goyal, 2012; Najafzadeh et al., 2016; Najafzadeh et al., 2018) that implements various learning algorithms. Weka is written in Java and developed at the University of Waikato, New Zealand. It is free software licensed under the GNU General Public License available from <http://www.cs.waikato.ac.nz/~ml>. For GP models we used GeneXprotool5 software. The results are shown in Tables 4 and 5.

**Table 2.** The proposed scenarios for rainfall-runoff modeling of Amameh Watershed

Scenario	Inputs
1	$P_t$
2	$ET_t$
3	$Rh_t$
4	$T_t$
5	$Q_{t-1}$
6	$P_t, P_{t-1}$
7	$P_t, Q_{t-1}$
8	$ET_t, Q_{t-1}$
9	$Rh_t, Q_{t-1}$
10	$T_t, Q_{t-1}$
11	$Q_{t-1}, Q_{t-2}$
12	$P_t, P_{t-1}, P_{t-2}$
13	$P_t, P_{t-1}, Q_{t-1}$
14	$P_t, Q_{t-1}, Q_{t-2}$
15	$ET_t, Q_{t-1}, Q_{t-2}$
16	$Rh_t, Q_{t-1}, Q_{t-2}$
17	$T_t, Q_{t-1}, Q_{t-2}$
18	$Q_{t-1}, Q_{t-2}, Q_{t-3}$
19	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}$
20	$P_t, P_{t-1}, Q_{t-1}, Q_{t-2}$
21	$P_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
22	$ET_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
23	$Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
24	$T_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
25	$P_t, T_t, ET_t, Rh_t$
26	$Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
27	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}, Q_{t-2}$
28	$P_t, P_{t-1}, Q_{t-1}, Q_{t-2}, Q_{t-3}$
29	$P_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
30	$ET_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$

$Q_{t=}$  represents the normalized daily discharge at the present time,  $Rh_{t=}$  represents the normalized daily relative humidity at the present time,  $ET_{t=}$  represents the normalized daily evapotranspiration at the present time,  $P_{t=}$  represents the normalized daily rainfall humidity at the present time,  $T_{t=}$  represents the normalized daily temperature at the present time, the indices t-1 to t-6 respectively refer to 1-day and 6-day lags and so on.

**Efficiency criteria**

The ability of models and scenarios to estimate daily discharge in Amameh Watershed was considered by applying the evaluation criteria. There are many performance criteria which have been used widely all over the world for rainfall-runoff relationship (Legates and McCabe, 1999; Dawson and Wilby, 2001; Huo et al., 2012; Moatamednia et al., 2015; Danandeh, 2018). The performance of all models in this article was evaluated by using a three statistical performance evaluation measures. These performance criteria included mean-square error (MSE), root-mean-square error (RMSE) and mean absolute error (MAE) (Srinivasulu and Jain, 2006; Danandeh et

al., 2013; Lu et al., 2018; Rezaie-Balf et al., 2019). RMSE measured the goodness of fit for high streamflow. In addition, it provided information about the predictive capabilities of the scenario. The error is the amount by which the value implied by the estimator differs from the target or quantity to be estimated (Danandeh Mehr et al., 2014). The above statistical parameters can be calculated using the following expressions (equations number 3 to 5).

$$MSE = \frac{\sum_{i=1}^N (Q_o - Q_e)^2}{N} \tag{3}$$

$$RMSE = \sqrt{\frac{\sum_{i=1}^N (Q_o - Q_e)^2}{N}} \tag{4}$$

$$MAE = \frac{1}{N} \sum_{i=1}^N |Q_o - Q_e| \tag{5}$$

where  $Q_o$ ,  $Q_e$  and  $N$  are measured and estimated values and number of data, respectively.

**Table 2.** Continued. The proposed scenarios for rainfall-runoff modeling of Amameh Watershed

Scenarios	Inputs
31	$Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
32	$T_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
33	$P_t, T_t, ET_t, Rh_t, Q_{t-1}$
34	$Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
35	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}, Q_{t-2}, Q_{t-3}$
36	$P_t, P_{t-1}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
37	$P_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
38	$ET_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
39	$Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
40	$T_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
41	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t$
42	$P_t, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}$
43	$Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
44	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
45	$P_t, P_{t-1}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
46	$P_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
47	$ET_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
48	$Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
49	$T_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
50	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}$
51	$P_t, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
52	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
53	$P_t, P_{t-1}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
54	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}$
55	$P_t, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
56	$P_t, P_{t-1}, P_{t-2}, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
57	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}$
58	$P_t, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
59	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}$
60	$P_t, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$
61	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}$
62	$P_t, P_{t-1}, P_{t-2}, T_t, ET_t, Rh_t, Q_{t-1}, Q_{t-2}, Q_{t-3}, Q_{t-4}, Q_{t-5}, Q_{t-6}$

$Q_t$ = represents the normalized daily discharge at the present time,  $Rh_t$ = represents the normalized daily relative humidity at the present time,  $ET_t$ = represents the normalized daily evapotranspiration at the present time,  $P_t$ = represents the normalized daily rainfall humidity at the present time,  $T_t$ = represents the normalized daily temperature at the present time, the indices t-1 to t-6 respectively refer to 1-day and 6-day lags and so on.

**Table 3.** Best parameter for GP model

Parameter	Value
Initial population (Program)	300
Linking function	Sum
Mutation rate	0.044
Inversion rate	0.01
One-point recombination rate	0.30
Two-point recombination rate	0.30
Gene recombination rate	0.10
Gene transposition rate	0.10
Maximum generation	1000

**Results and Discussion**

As already mentioned, 62 scenarios (see Table 2) were considered for rainfall-runoff modeling of Amameh Watershed using GP and M5 model tree. The best values for various parameters in GP were

obtained using trial-and-error to minimize the RMSE during the model fitting process shown in Table 3. The results of GP and M5 model tree are presented in Tables 4 and 5.



**Table 4.** The results of GP in Amameh Watershed

Scenarios	Training			Testing		
	MAE	RMSE	MSE	MAE	RMSE	MSE
1	0.015	0.078	0.006	0.016	0.089	0.008
2	0.015	0.079	0.006	0.016	0.090	0.008
3	0.015	0.084	0.007	0.017	0.095	0.009
4	0.015	0.077	0.006	0.016	0.089	0.007
5	0.014	0.068	0.005	0.015	0.078	0.006
6	0.015	0.076	0.006	0.016	0.088	0.007
7	0.013	0.067	0.004	0.015	0.077	0.005
8	0.013	0.067	0.004	0.015	0.077	0.005
9	0.013	0.067	0.004	0.015	0.077	0.005
10	0.013	0.067	0.004	0.015	0.077	0.005
11	0.014	0.068	0.005	0.015	0.078	0.006
12	0.014	0.071	0.006	0.016	0.086	0.007
13	0.012	0.055	0.003	0.013	0.064	0.004
14	0.012	0.059	0.003	0.014	0.068	0.004
15	0.012	0.059	0.003	0.014	0.068	0.004
16	0.012	0.059	0.003	0.014	0.068	0.004
17	0.012	0.059	0.003	0.014	0.068	0.004
18	0.014	0.068	0.005	0.015	0.078	0.006
19	0.013	0.067	0.004	0.015	0.077	0.005
20	0.012	0.057	0.003	0.013	0.065	0.004
21	0.012	0.057	0.003	0.013	0.064	0.004
22	0.012	0.057	0.003	0.013	0.065	0.004
23	0.012	0.057	0.003	0.013	0.066	0.004
24	0.011	0.048	0.002	0.013	0.060	0.003
25	0.015	0.075	0.006	0.016	0.087	0.007
26	0.014	0.069	0.005	0.015	0.079	0.006
27	0.012	0.055	0.003	0.013	0.063	0.004
28	0.012	0.055	0.003	0.013	0.064	0.004
29	0.012	0.058	0.003	0.013	0.066	0.004
30	0.012	0.058	0.003	0.013	0.067	0.004
31	0.012	0.058	0.003	0.013	0.067	0.004
32	0.012	0.057	0.003	0.013	0.066	0.004
33	0.012	0.057	0.003	0.013	0.065	0.004
34	0.014	0.069	0.005	0.015	0.079	0.006

As can be seen from the Tables 4 and 5 in the training period, the GP method achieved the best MSE, RMSE, and MAE evaluation criteria of 0.001, 0.031, and 0.0009, for scenario 54. Whilst the M5 method provided the best results with the least error, namely 0.057, 0.197 and 0.039, respectively for MSE, RMSE, and MAE for model 54. It can be observed from the testing period results, both GP and M5 methods have high MSE, RMSE and MAE errors in this period, namely 0.009, 0.032 and 0.001 for GP and 0.085, 0.255 and 0.065 for M5 model respectively.

Here we developed three models. The first model contained only meteorological inputs, the second model consisted of hydrometric and the third model used both

meteorological and hydrometric variables as inputs. According to error analysis of the test set model, in which we only used meteorological variable, we had higher errors than the one with hydrometric variable as input. This finding showed that antecedent discharge of Amameh Watershed had more effect on the results. Out of these parameters, relative humidity, evapotranspiration, rainfall and temperature had the most errors, respectively. Therefore the least and the most effective meteorological variable in this watershed were relative humidity and temperature. The MSE, RMSE and MAE of GP were 0.015, 0.084 and 0.007 for training and 0.017, 0.095 and 0.009 for testing, respectively. Additionally, these error terms

for M5 model tree were 0.620, 0.760 and 0.553 for testing, respectively. 0.578 for training and 0.594, 0.744 and

**Table 4.** Continued. The results of GP in Amameh Watershed testing

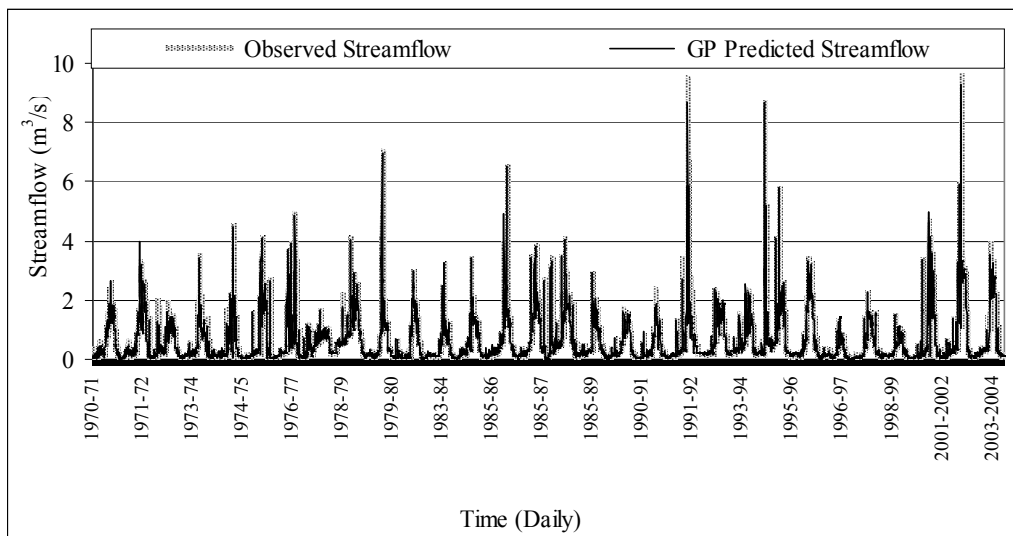
Scenarios	Training			Testing		
	MAE	RMSE	MSE	MAE	RMSE	MSE
35	0.011	0.046	0.002	0.011	0.048	0.002
36	0.011	0.048	0.002	0.013	0.060	0.003
37	0.013	0.064	0.004	0.014	0.072	0.005
38	0.013	0.064	0.004	0.014	0.072	0.005
39	0.013	0.064	0.004	0.014	0.074	0.005
40	0.013	0.063	0.004	0.014	0.071	0.005
41	0.014	0.070	0.005	0.015	0.085	0.007
42	0.011	0.046	0.002	0.011	0.048	0.002
43	0.014	0.069	0.005	0.015	0.084	0.007
44	0.011	0.048	0.002	0.012	0.058	0.003
45	0.011	0.048	0.002	0.012	0.058	0.003
46	0.013	0.066	0.004	0.015	0.076	0.005
47	0.013	0.066	0.004	0.015	0.076	0.005
48	0.013	0.066	0.004	0.015	0.076	0.005
49	0.013	0.065	0.004	0.015	0.075	0.005
50	0.011	0.046	0.002	0.011	0.047	0.002
51	0.011	0.046	0.002	0.011	0.046	0.002
52	0.011	0.048	0.002	0.012	0.057	0.003
53	0.011	0.048	0.002	0.012	0.058	0.003
54	0.001	0.031	0.009	0.009	0.032	0.001
55	0.002	0.047	0.011	0.012	0.050	0.002
56	0.002	0.048	0.011	0.012	0.057	0.003
57	0.002	0.045	0.010	0.010	0.045	0.002
58	0.002	0.047	0.011	0.012	0.055	0.003
59	0.002	0.046	0.011	0.011	0.048	0.002
60	0.002	0.048	0.011	0.012	0.057	0.003
61	0.002	0.047	0.011	0.011	0.049	0.002
62	0.002	0.047	0.011	0.012	0.055	0.003

The results furthermore showed that more than one variable as input affected runoff so that the combinations of meteorological and hydrometric variables were necessary. According to the results in Tables 4 and 5, the best model was number 54 in which we used the concurrent rainfall, one and two antecedent rainfall, the concurrent temperature and one and two antecedent runoff, the current evapotranspiration and relative humidity. To prevent overgrowing, the maximum size of the program was restricted (Brameier and Banzhaf, 2001; Gorbani et al., 2010; Danandeh Mehr, 2018). The maximum generation was 1000 and the function sets were defined by modeler based on two sets of mathematical functions. The first set was the software default consisting of 11 functions such as sin, cos, tang and

cotg and the other was four basic mathematical operations {+, -, \* and /} and power function. According to the results, the basic mathematical operations plus power were better than those of software default. The results showed that the rainfall-runoff relationship is complex and non-linear, and its estimation using these functions caused accuracy reduction (Danandeh Mehr, 2018). The GP model due to its high efficiency enables estimation of non-linear relationship with the basic mathematical operations. These findings were consistent with those of other researchers (Khu et al., 2001; Whigham and Crapper, 2001; Liong et al., 2002; Jayawardena et al., 2005; Ustoorikar and Deo, 2008; Guven, 2009; Danandeh Mehr et al., 2013; Danandeh Mehr et al., 2014).

**Table 5.** The results of M5 in Amameh Watershed

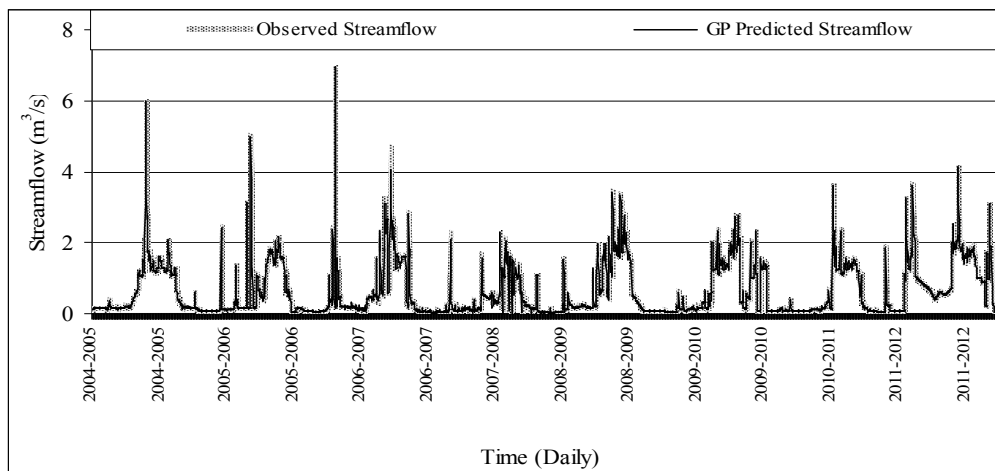
Scenarios	Training			Testing		
	MAE	RMSE	MSE	MAE	RMSE	MSE
1	0.606	0.753	0.567	0.576	0.733	0.538
2	0.612	0.756	0.572	0.588	0.739	0.546
3	0.620	0.760	0.578	0.594	0.744	0.553
4	0.592	0.742	0.550	0.566	0.728	0.530
5	0.124	0.354	0.125	0.140	0.382	0.146
6	0.573	0.731	0.535	0.558	0.724	0.524
7	0.100	0.285	0.081	0.110	0.329	0.108
8	0.100	0.290	0.084	0.110	0.330	0.109
9	0.110	0.316	0.100	0.110	0.335	0.112
10	0.100	0.285	0.081	0.110	0.329	0.108
11	0.110	0.332	0.110	0.122	0.352	0.124
12	0.472	0.642	0.412	0.514	0.704	0.495
13	0.093	0.263	0.069	0.110	0.319	0.102
14	0.096	0.266	0.071	0.110	0.322	0.104
15	0.096	0.266	0.071	0.110	0.322	0.104
16	0.096	0.266	0.071	0.110	0.322	0.104
17	0.096	0.266	0.071	0.110	0.321	0.103
18	0.110	0.332	0.110	0.126	0.356	0.127
19	0.100	0.283	0.080	0.110	0.327	0.107
20	0.093	0.263	0.069	0.110	0.319	0.102
21	0.093	0.263	0.069	0.110	0.319	0.102
22	0.093	0.263	0.069	0.110	0.319	0.102
23	0.095	0.265	0.070	0.110	0.321	0.103
24	0.091	0.261	0.068	0.110	0.316	0.100
25	0.506	0.696	0.484	0.520	0.710	0.504
26	0.130	0.363	0.132	0.140	0.389	0.151
27	0.091	0.261	0.068	0.110	0.316	0.100
28	0.093	0.263	0.069	0.110	0.319	0.102
29	0.095	0.265	0.070	0.110	0.321	0.103
30	0.095	0.265	0.070	0.110	0.321	0.103
31	0.095	0.265	0.070	0.110	0.321	0.103
32	0.095	0.265	0.070	0.110	0.321	0.103
33	0.093	0.263	0.069	0.110	0.319	0.102
34	0.134	0.374	0.140	0.152	0.412	0.170



**Figure 3.** Observed and predicted flows based on genetic programming (GP) during training period for Amameh Watershed

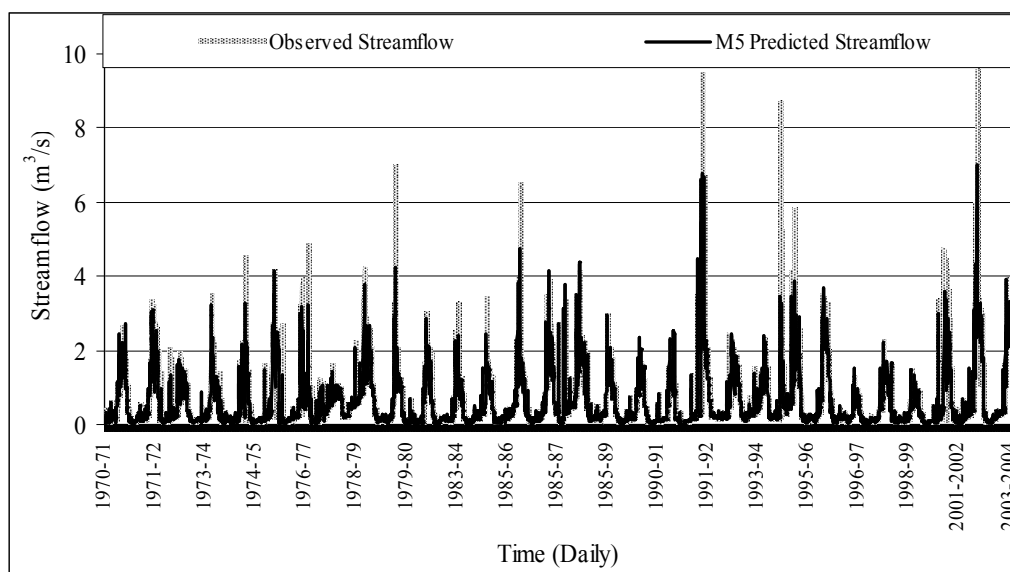
**Table 5.** Continued. The results of M5 in Amameh watershed

Scenarios	Training			Testing		
	MAE	RMSE	MSE	MAE	RMSE	MSE
35	0.083	0.253	0.064	0.110	0.311	0.097
36	0.091	0.261	0.068	0.110	0.316	0.100
37	0.098	0.268	0.072	0.110	0.322	0.104
38	0.098	0.268	0.072	0.110	0.324	0.105
39	0.100	0.272	0.074	0.110	0.324	0.105
40	0.096	0.266	0.071	0.110	0.322	0.104
41	0.464	0.634	0.402	0.506	0.696	0.484
42	0.085	0.255	0.065	0.110	0.311	0.097
43	0.140	0.385	0.148	0.158	0.418	0.175
44	0.091	0.261	0.068	0.110	0.316	0.100
45	0.091	0.261	0.068	0.110	0.316	0.100
46	0.100	0.274	0.075	0.110	0.324	0.105
47	0.100	0.279	0.078	0.110	0.326	0.106
48	0.100	0.281	0.079	0.110	0.326	0.106
49	0.100	0.274	0.075	0.110	0.324	0.105
50	0.083	0.253	0.064	0.110	0.311	0.097
51	0.081	0.251	0.063	0.110	0.311	0.097
52	0.089	0.259	0.067	0.110	0.315	0.099
53	0.091	0.261	0.068	0.110	0.316	0.100
54	0.057	0.197	0.039	0.085	0.255	0.065
55	0.087	0.257	0.066	0.110	0.313	0.098
56	0.089	0.259	0.067	0.110	0.316	0.100
57	0.081	0.251	0.063	0.110	0.310	0.096
58	0.087	0.257	0.066	0.110	0.315	0.099
59	0.085	0.255	0.065	0.110	0.311	0.097
60	0.089	0.259	0.067	0.110	0.315	0.099
61	0.085	0.255	0.065	0.110	0.311	0.097
62	0.087	0.257	0.066	0.110	0.315	0.099

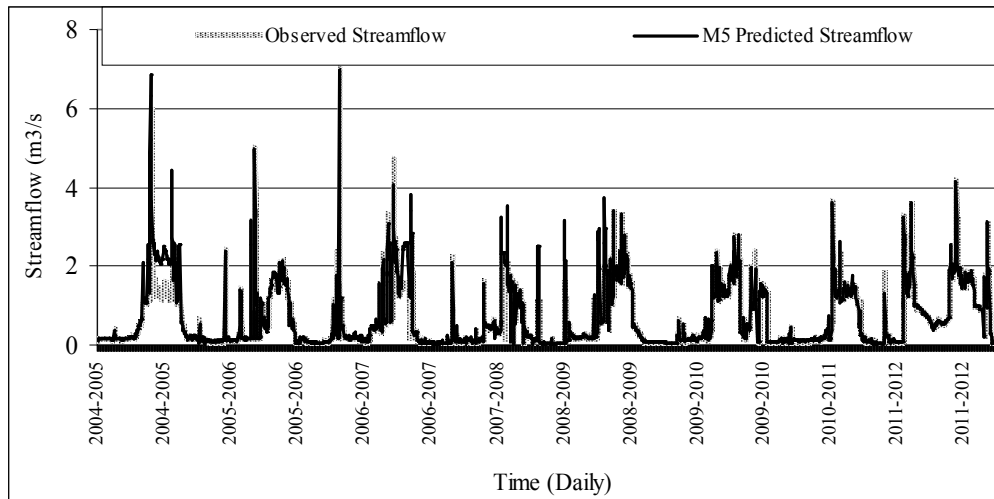
**Figure 4.** Observed and predicted flows based on genetic programming (GP) during testing period for Amameh Watershed

**Table 6.** Relationship between variables in model number 54 provided by M5 model tree

Row	Rules
1	If $Q_{t-1} \leq 0.59$ $Q_{t-2} > 0.16$ $Q_{t-1} > 0.32$ , Then $Q_t = 0.003 * P_t - 0.02 * P_{t-1} - 0.03 * P_{t-2} + 0.001 * Rh_t + 1.05 * Q_{t-1} - 0.12 * Q_{t-2} - 0.02$ [1526/24.315%]
2	If $Q_{t-1} \leq 0.59$ $Q_{t-2} \leq 0.17$ $Q_{t-1} \leq 0.12$ $Q_{t-1} > 0.09$ , Then $Q_t = 0.0001 * P_t - 0.05 * P_{t-1} - 0.004 * P_{t-2} + 0.05 * T_t - 0.001 * ET_t + 0.01 * Rh_t + 0.02 * Q_{t-1} + 0.01 * Q_{t-2} + 0.12$ [1359/4.415%]
3	If $Q_{t-1} \leq 0.59$ $Q_{t-1} \leq 0.18$ $Rh_t \leq 51.5$ , Then $Q_t = 0.0001 * P_t - 0.004 * P_{t-1} - 0.003 * P_{t-2} + 0.08 * T_t - 0.001 * ET_t + 0.001 * Rh_t + 0.92 * Q_{t-1} + 0.012 * Q_{t-2} + 0.009$ [1636/4.382%]
4	If $Q_{t-1} \leq 0.59$ $Q_{t-1} > 0.18$ $Q_{t-1} \leq 0.23$ , Then $Q_t = 0.004 * P_t - 0.001 * P_{t-1} - 0.003 * P_{t-2} + 0.05 * T_t - 0.004 * ET_t + 0.003 * Rh_t + 0.66 * Q_{t-1} + 0.13 * Q_{t-2} + 0.034$ [1016/7.214%]
5	If $Q_{t-1} \leq 0.60$ $Q_{t-1} > 0.2$ $P_t \leq 0.75$ , Then $Q_t = 0.0001 * P_t - 0.006 * P_{t-1} - 0.001 * P_{t-2} + 0.004 * T_t - 0.002 * ET_t + 0.0001 * Rh_t + 0.81 * Q_{t-1} + 0.10 * Q_{t-2} + 0.03$ [980/5.342%]
6	If $Q_{t-1} > 1.08$ $Q_{t-1} \leq 2.24$ $Q_{t-1} \leq 1.60$ $P_{t-1} \leq 1.65$ , Then $Q_t = 0.013 * P_t - 0.007 * P_{t-1} - 0.004 * P_{t-2} + 0.004 * T_t - 0.0002 * ET_t + 0.001 * Rh_t + 0.88 * Q_{t-1} + 0.02 * Q_{t-2} + 0.14$ [814/29.142%]
7	If $Q_{t-1} > 1.08$ $Q_{t-1} \leq 2.30$ $Q_{t-1} > 1.60$ , Then $Q_t = 0.013 * P_t - 0.007 * P_{t-1} - 0.0002 * P_{t-2} + 0.002 * T_t + 0.0001 * Rh_t + 0.63 * Q_{t-1} + 0.23 * Q_{t-2} + 0.24$ [716/40.251%]
8	If $Q_{t-1} \leq 1.08$ $Q_{t-2} \leq 0.31$ $Q_{t-1} \leq 0.16$ $P_t \leq 0.85$ , Then $Q_t = 0.001 * P_t + 0.0001 * P_{t-1} - 0.0004 * P_{t-2} + 0.0001 * T_t - 0.0002 * ET_t + 0.0001 * Rh_t + 0.94 * Q_{t-1} + 0.012 * Q_{t-2} + 0.01$ [435/3.573%]
9	If $Q_{t-1} \leq 1.07$ $Q_{t-2} > 0.305$ $Rh_t \leq 51.5$ $Q_{t-1} > 0.77$ , Then $Q_t = 0.0003 * P_t + 0.0001 * P_{t-1} - 0.001 * P_{t-2} - 0.003 * T_t - 0.0004 * ET_t + 0.0001 * Rh_t + 0.93 * Q_{t-1} - 0.09 * Q_{t-2} + 0.173$ [435/11.755%]
10	If $Q_{t-1} > 1.075$ $Q_{t-1} > 1.95$ , Then $Q_t = 0.0001 * P_t - 0.006 * P_{t-1} - 0.008 * P_{t-2} + 0.0001 * T_t + 0.008 * Rh_t + 0.442 * Q_{t-1} + 0.12 * Q_{t-2} + 0.811$ [411/82.488%]
11	If $Q_{t-2} \leq 0.305$ , Then $Q_t = 0.004 * P_t + 0.002 * P_{t-1} - 0.003 * P_{t-2} + 0.002 * T_t - 0.001 * ET_t + 0.0001 * Rh_t + 0.498 * Q_{t-1} + 0.2354 * Q_{t-2} + 0.0392$ [554/31.238%]
12	If $Q_{t-1} \leq 0.99$ $Rh_t > 51.5$ $Q_{t-1} > 0.655$ , Then $Q_t = 0.004 * P_t - 0.0002 * P_{t-1} + 0.009 * T_t - 0.025 * ET_t + 0.0001 * Rh_t + 1.11 * Q_{t-1} + 0.025 * Q_{t-2} - 0.084$ [378/44.009%]
13	If $Q_{t-1} > 0.88$ $P_{t-2} \leq 7.5$ , Then $Q_t = 0.008 * P_t + 0.001 * P_{t-1} - 0.0007 * P_{t-2} + 0.002 * T_t - 0.003 * ET_t + 0.0002 * Rh_t + 0.56 * Q_{t-1} + 0.0782 * Q_{t-2} + 0.45$ [219/79.17%]
14	If $Rh_t \leq 51.5$ $Q_{t-1} \leq 0.68$ , Then $Q_t = 0.001 * P_t - 0.0002 * P_{t-2} + 0.001 * T_t - 0.001 * ET_t + 0.0002 * Rh_t + 0.99 * Q_{t-1} - 0.034 * Q_{t-2} - 0.002$ [129/19.56%]
15	If $P_{t-1} \leq 4.75$ $Rh_t \leq 51.5$ , Then $Q_t = 0.002 * P_t - 0.001 * P_{t-2} - 0.003 * T_t + 0.0003 * Rh_t + 0.679 * Q_{t-1} + 0.104 * Q_{t-2} + 0.20$ [139/19.388%]
16	If $T_t \leq 3.15$ , Then $Q_t = 0.003 * P_t + 0.004 * T_t - 0.01 * ET_t + 0.001 * Rh_t + 0.63 * Q_{t-1} + 0.13 * Q_{t-2} + 0.069$ [108/17.38%]
17	$Q_t = 0.044 * P_t + 0.02 * Rh_t + 1.140 * Q_{t-2} - .47$ [89/65.448%]



**Figure 5.** Observed and predicted flows based on M5 model tree during training period for Amameh Watershed



**Figure 6.** Predicted and observed flow during testing period by M5 for Amameh Watershed

According to Table 6, 17 rules suggested by M5 model tree could solve this problem. Several advantages such as user friendliness, fast training process, understandable results and simple and linear equations could be noted for this model. It is important to mention that although the equations governing the M5 model were not really physically interpretable, but they allowed the modelers to quickly check the predicted streamflow as reported before by Solomatine, and Dulal (2003).

Figures 3 to 6 show that although GP method could simulate and predict streamflow with low errors, but both methods of GP and M5 underestimate the Amameh Watershed streamflow, and this is especially so for M5 method in training period. We showed that flow peaks of the study datasets often led to poor performances. Furthermore, high accuracy of prediction could not be achieved by this model (Wu and Chen, 2005; Ni et al., 2010; Danandeh Mehr et al., 2014; Dnandeh Mehr, 2018). In the case of Hao et al. (2006), the data used for prediction were much smoother and this could have had something to do with their good forecasting. The final mathematical relationship obtained using GP for Amameh Watershed is expressed as follows:

$$Q_t = (Q_{t-2}/Q_{t-1})((RH_t^{Q_{t-1}} * 0.92P_t)) + (Q_{t-1} - (0.02P_{t-1})0.06) + 0.089T_t + 0.089P_{t-2} - ET_t \quad (6)$$

The results showed that the GP model had good simulation and prediction due to low errors during training and testing periods, respectively. This finding verified the results of Selle and Muttill (2011) who studied the GP in southeastern Australia. It is believed that GP could be used to get insight into the dominant events of hydrological cycles, practically.

### Conclusion

The experimental Amameh Watershed in Tehran Province, Iran was subjected to rainfall-runoff modeling using two approaches of GP and M5 model tree under 62 different scenarios. The results showed better performance of GP in almost all scenarios for daily streamflow prediction. It was also found that GP had appropriate potential for solving complex and nonlinear hydrological modeling problems. The results indicated that the relative humidity (Rh) had the least sensitivity in streamflow prediction among other meteorological variables i.e., rainfall (P), mean air temperature (T), and evapotranspiration (ET). Furthermore, the performances of all developed scenarios were assessed using standard statistical

performance evaluation measures such as MSE, RMSE and MAE. However, we recommend error analysis for varying

ranges of flow such as low, medium and high streamflow.

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